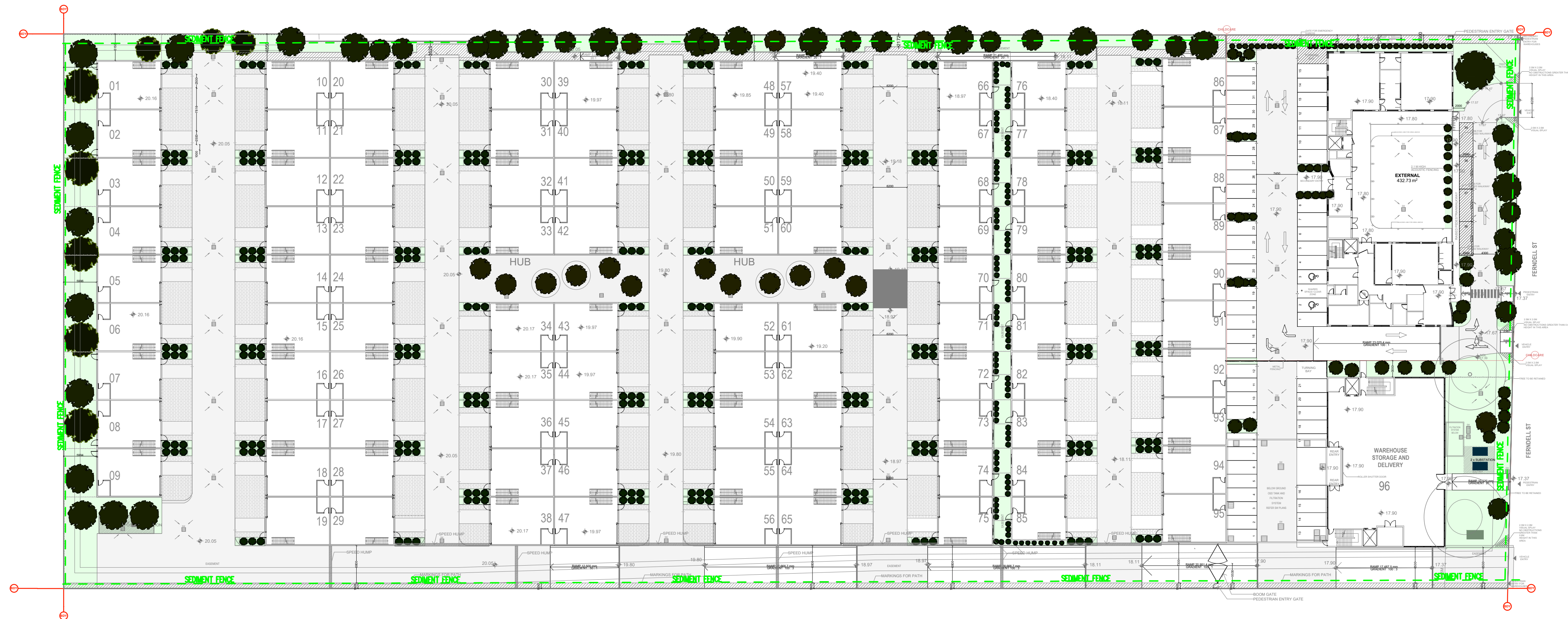
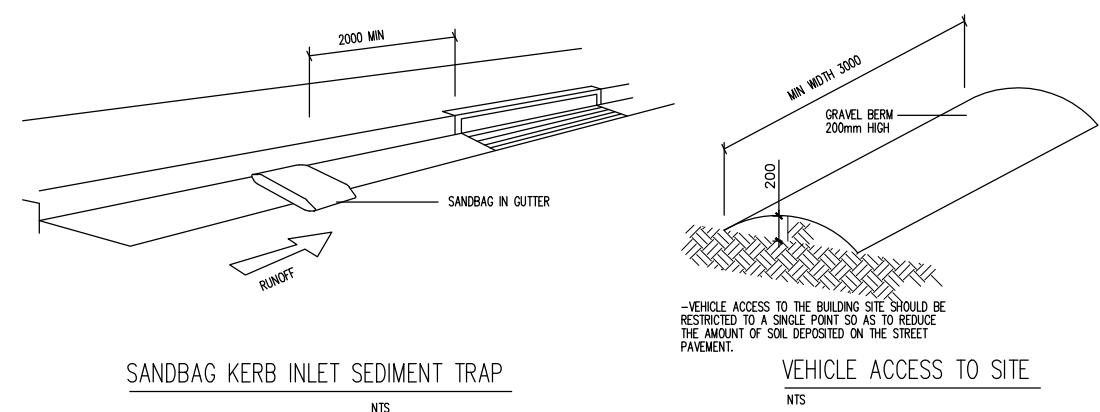
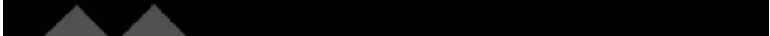
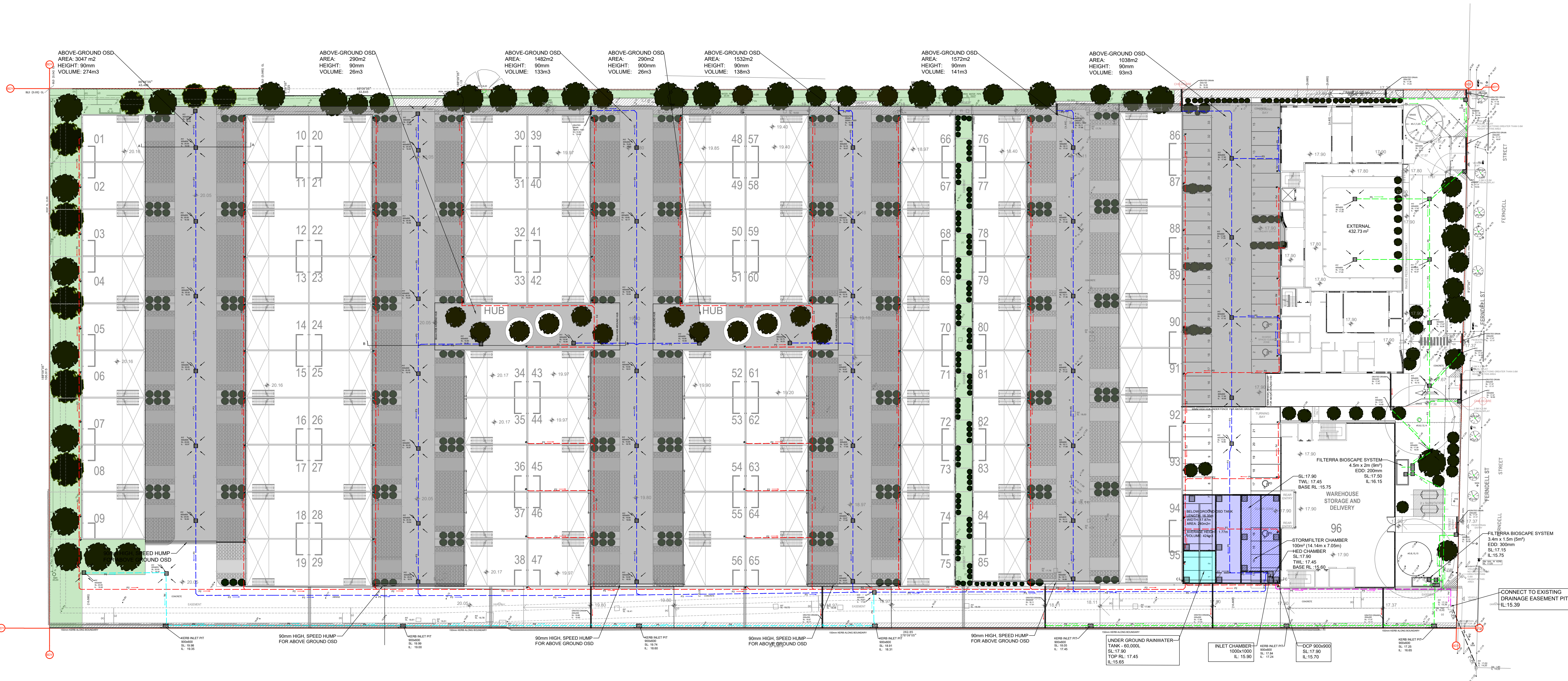


NOTES

-
- STAR PICKET
- GEOTEXTILE FILTER FABRIC BARRIERS DOWN WITH STARS TO BE KEPT TO ELATION FENCE LINE
- RUNOUT
- GEOTEXTILE FABRIC SECURELY FIXED TO FENCE
- 3x2.5 WIRES AT 150 CENTRES
- STAR PICKET
- (TWO GEOTEXTILE FABRIC 200 MM WIDE STRIPS)
- END OF SILTATION FENCE TO BE RETURNED UP LEAST TO PREVIOUS STRAP
- 400
- 300 MM
- PIT
- GEOTEXTILE FILTER PIT SURROUND
- SILTATION FENCE DETAIL



Designed by:	E.C.	ERTA Z H, CHOWDHURY MIEAust., CPEng. NER, Reg. No. 2214897	Australiawide Consulting Services P/L Civil & Structural Engineering and Project Management 1369B Canterbury Rd, Punchbowl, NSW 2196 Mobile: 0420710548 Email: ertaz1@hotmail.com		Architect: 	Client: 	Project North: 	Project: PROPOSED MIXED USE DEVELOPMENT AT 46-52 FERNDELL STREET, SOUTH GRANVILLE, NSW 2142	Job: STORMWATER MANAGEMENT DRAWING PACKAGE FOR DA	Date: 08-05-2024
Drawn by:	A.A.								Title: SEDIMENT CONTROL PLAN	Scale @A1: 1:500
Revision:	01									Drg No. SW02



LEGEND:

- "DP" DOWNPIPE - Ø150MM UPVC
- "P2" Ø200mm UPVC @ MIN 1% GRADE (DP TO RWT)
- "P3" Ø150mm UPVC @ MIN 0.5% GRADE (SURFACE PITS TO OSD)
- "P4" Ø300mm UPVC @ MIN 1% GRADE (OSD TO EASEMENT PIT)
- "P5" Ø150mm UPVC @ MIN 1% GRADE (SURFACE PITS / KIP TO P4 VIA FILTERRA)
- "P6" Ø150mm UPVC @ MIN 1% GRADE (KERB INLET PITS TO OSD)
- "P7" Ø100mm UPVC @ MIN 1% GRADE (RAINWATER TANK OVERFLOW TO STORMFILTER CHAMBER)
- "P8" EXISTING Ø900mm PIPE (EASEMENT PIT TO COUNCIL KIP)
- ABOVE GROUND OSD AREA
- BELOW GROUND OSD AREA
- BELOW GROUND RAINWATER TANK
- LANDSCAPE OSD BYPASS

GENERAL NOTES:

- ALL WORK SHALL BE IN ACCORDANCE WITH THE BUILDING CODE OF AUSTRALIA AND AS3500.3, COUNCIL'S STANDARD SPECIFICATIONS CODES.
- THIS PLAN IS TO BE READ IN CONJUNCTION WITH THE ARCHITECTURAL, LANDSCAPE AND STRUCTURAL PLANS.
- MINIMUM GRADES FOR ALL PIPE SHOULD BE 1%, UNO.
- DIRECT SURFACE FLOW TO ALL GRATED SURFACE INLET PITS.
- ALL DESIGN LEVELS SHOWN ON PLAN SHALL BE VERIFIED ON SITE PRIOR TO THE COMMENCEMENT OF ANY WORK.
- ANY DISCREPANCIES OR OMISSIONS SHALL BE REFERRED TO THE DESIGN ENGINEER FOR RESOLUTION.
- ALL SURFACE DRAINAGE PITS & GRATED STRIP DRAINS TO HAVE OCEANGUARD INSERTS. REFER TO OCEANGUARD SPECIFICATION DRAWINGS BY OCEANPROTECT & MUSIC MODEL FOR DETAILS.
- ALL EXISTING GRATED PITS TO BE CLOSED TO PREVENT SURFACE RUNOFF TO THESE PITS. ALL SURFACE DRAINAGE TO BE DRAINED TO PROPOSED DRAINAGE PITS TO COMPLY WITH WSUD POLICY

OSD NOTES:

TOTAL SITE AREA: 2.657 H
AREA DRAINING TO OSD: 2.2665 H (85.3%)
BYPASS AREA: 3905m² (14.7%)

TOTAL OSD VOLUME REQUIRED AS PER COUNCIL:
(REFER TO DRAINAGE DESIGN SUMMARY):
552.42 X 2.2665 = 1252.06m³

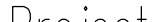
ABOVE GROUND OSD VOLUME PROVIDED - 829m³
BELOW GROUND OSD VOLUME PROVIDED - 424m³
TOTAL OSD VOLUME PROVIDED - 1253m³

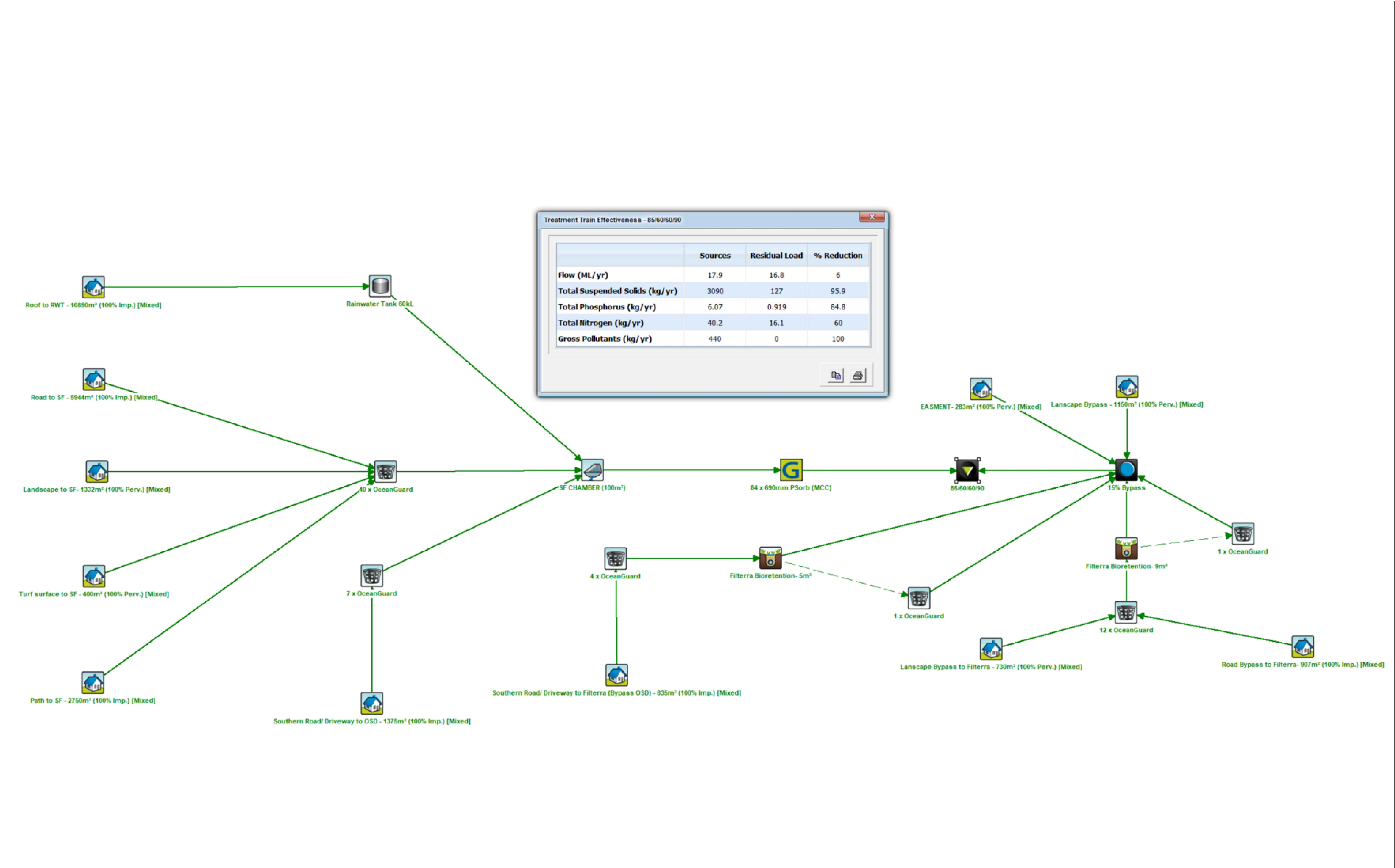
Designed by:	E.C.	ERTAZ H, CHOWDHURY MIEAust., CPEng. NER, Reg. No. 2214897	Australiawide Consulting Services P/L Civil & Structural Engineering and Project Management 1369B Canterbury Rd, Punchbowl, NSW 2196 Mobile: 0420710548 Email: ertaz1@hotmail.com	Architect: GHAZI AL ALI ARCHITECT	Client: Trademaster trademaster.com.au	Project North: N	Project: PROPOSED MIXED USE DEVELOPMENT AT 46-52 FERNDILL STREET, SOUTH GRANVILLE, NSW 2142	Job:	Date:
Drawn by:	A.A.							STORMWATER MANAGEMENT DRAWING PACKAGE FOR DA	08-05-2024
Revision:	01							Title: SITE DRAINAGE PLAN	Scale @A1: 1:400 Drg No. SW06

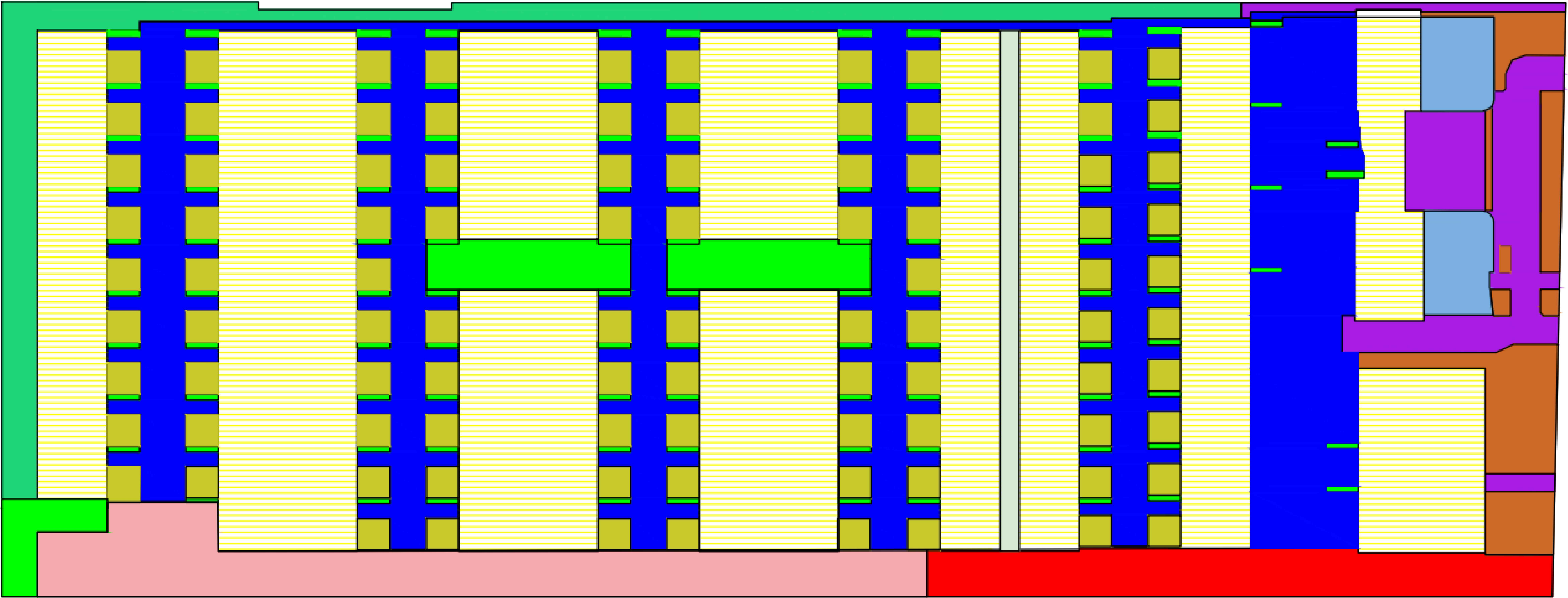


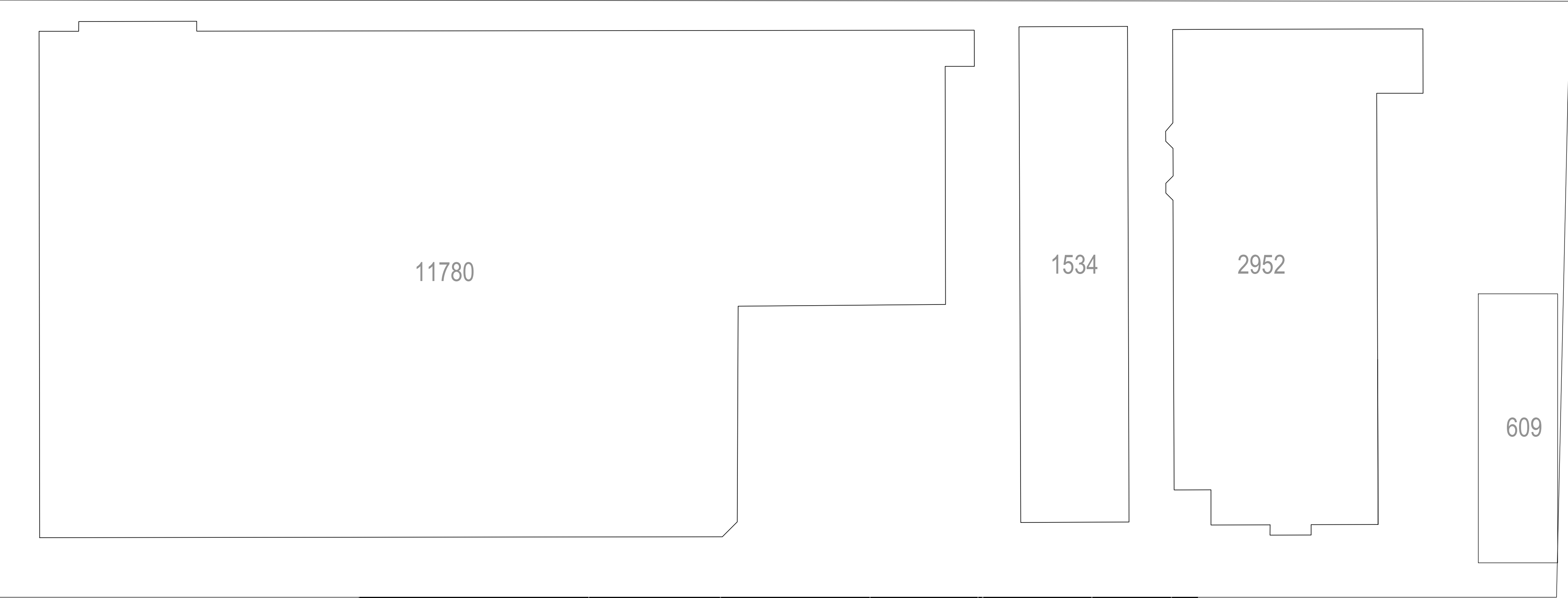
STRUCTURAL DETAILS OF UNDERGROUND RAINWATER & ON-SITE DETENTION TANK (C-C)
1:30



Designed by:	E.C.	ERTA Z H, CHOWDHURY MIEAust., CPEng, NER, Reg. No. 2214897	Australiawide Consulting Services P/L Civil & Structural Engineering and Project Management 1369B Canterbury Rd, Punchbowl, NSW 2196 Mobile: 0420710548 Email: ertaz1@hotmail.com	Architect: 	Client: 	Project North: 	Project: PROPOSED MIXED USE DEVELOPMENT AT 46-52 FERNDILL STREET, SOUTH GRANVILLE, NSW 2142	Job: STORMWATER MANAGEMENT DRAWING PACKAGE FOR DA	Date: 08-05-2024
Drawn by:	A.A.							Scale @A1: 1:30	
Revision:	01							Title: OSD DETAILS	Drg No. SW07







PRE DEVELOPMENT

TOTAL SITE AREA : 26556

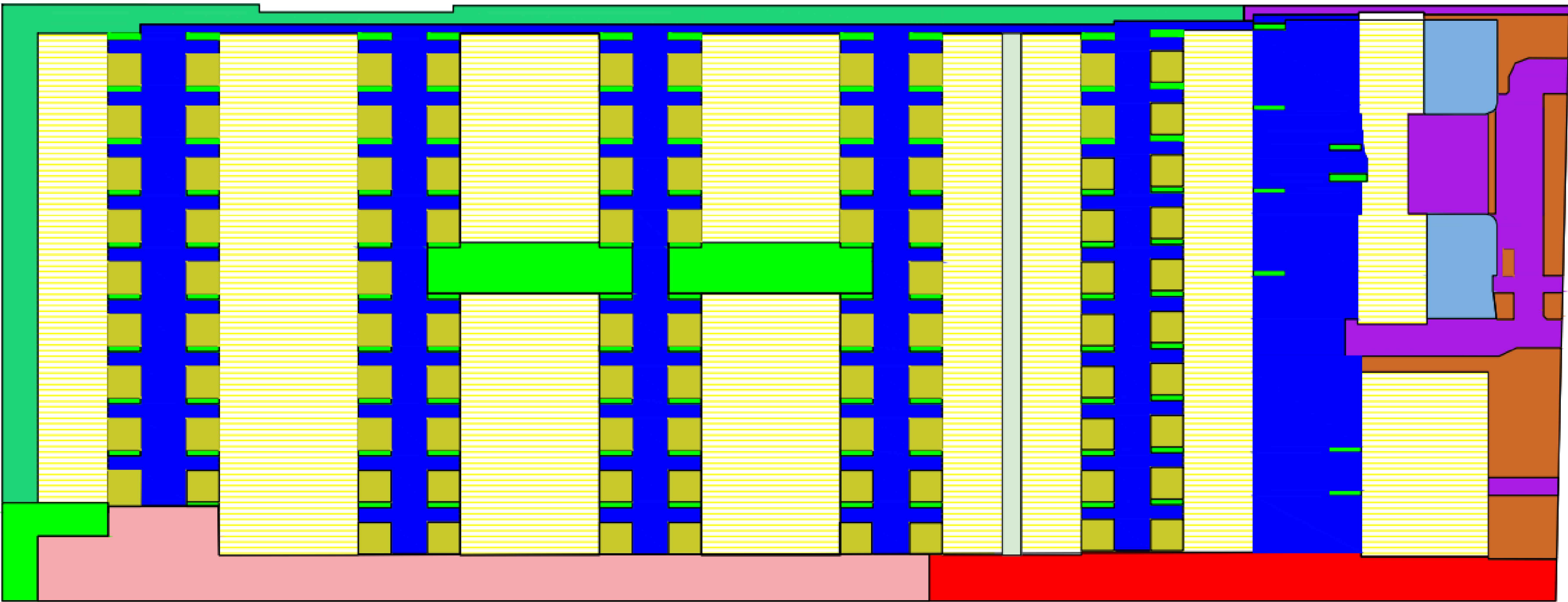
TOTAL GREEN AREA : 609 SQM

BUILDING FOOTPRINT AREA: 16266 SQM

DRIVEWAY AND CARPARK AREA: 8675 SQM

AREA DRAINING TO PIPE: 25500 SQM

PIPE CAPACITY CALCULATION: PRE DEVELOPMENT				
Pipe capacity calculation for 900 mm dia pipe(ARI 1 to 5)				
Address:	otal Building footprin	Green Area	Total Drive way / car park	Source
46-52 Ferndell St, South Granville	16266	609	8675 sqm	Arch, Plan
Total Area draining to Pipe (m ²) =		25550.00		
Runoff Coefficient, C (5 yrs) =		0.86		
Rain fall Data for the site from BOM				
Storm Duration T (min)	I 5 yrs (mm/hr)	V of Hydrograph (m ³)	Inflow Peak discharge (L/s)	
1	185	67.75	1129.17	
2	143	104.74	872.82	
3	135	148.32	823.99	
4	129	188.97	787.37	
5	124	227.05	756.85	
10	101	369.88	616.46	
15	84.2	462.53	513.92	
20	72.1	528.08	440.07	
25	63.2	578.62	385.75	
30	56.3	618.54	343.63	
45	43	708.63	262.46	
60	35.3	775.65	215.46	
90	26.7	880.02	162.97	
120	22	966.81	134.28	
180	17	1120.62	103.76	
Q	1129.17	L/s		
V	6	m/s (should be from 0.6 to 6 m/s)		
Q=VA				
A (Full)	0.188194676			
d Full)	489.5067962	mm	< 900 mm	Existing pipe from site to Council
			OK	
Mannings n for RC pipe	0.013			
Existing pipe dia	900	mm		
Pipe length	11.4	m		
Upstream IL	15.36	m		
Downstream IL	14.71	m		
Hydraulic Radius, R (Full Flow)	225	mm		
Slope, S	0.057017544			
Pipe flow Velocity	6.794892325	m/s, By Manning's formula		



- ROOF TO RWT: 10850m²

LANDSCAPE BYPASS 1150m²

EASMENT 283m²
- PATH TO SF CHAMBER: 2750m²

SOUTHERN ROAD TO OSD 1375m²

SOUTHERN ROAD BYPASS TO FILTERRA 835m²
- LANDSCAPE TO SF CHAMBER: 1332m²

ROAD/PATH BYPASS TO FILTERRA: 907m²

LANDSCAPE BYPASS TO FILTERRA: 730m²
- ROAD/FOOTPATH TO SF CHAMBER: 5944m²

TURF SURFACE TO SF CHAMBER: 400m²

TOTAL AREA: 26556m²

POST DEVELOPMENT

TOTAL SITE AREA : 26556

TOTAL GREEN AREA : 3495 SQM

ROOF AREA: 10850 SQM

DRIVEWAY, CARPARK, PATH, TURF AREA: 12211 SQM

AREA DRAINING TO PIPE: 26556 SQM

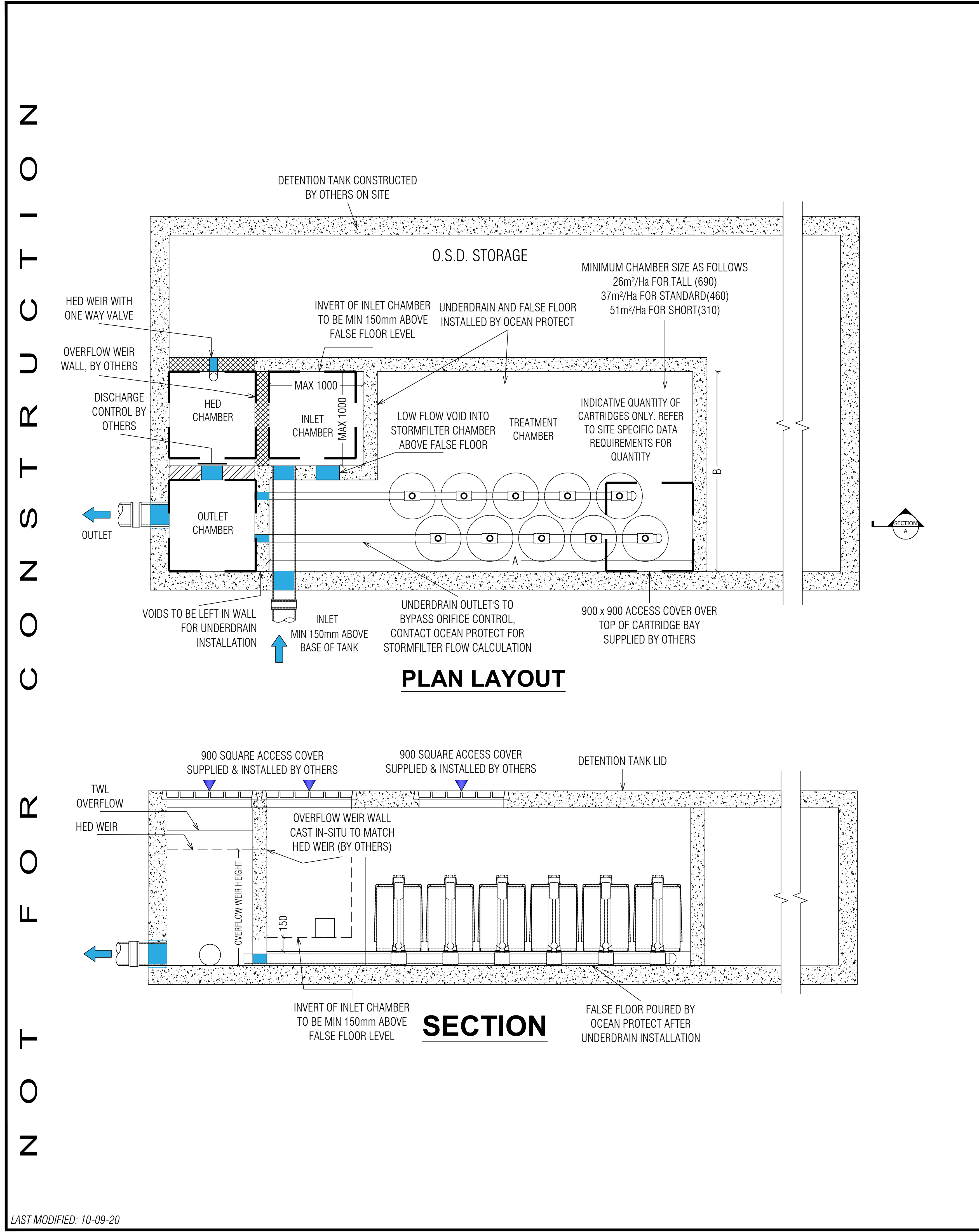
PIPE CAPACITY CALCULATION: POST DEVELOPMENT					
Pipe capacity calculation for 900 mm dia pipe(ARI 1 to 5)					
Address:	Total Building footprint	Green Area	Driveway, carpar, path, turf	Source	
46-52 Ferndell St, South Granville	10850	3495	12211	sqm	Arch. Plan
Total Area draining to Pipe (m²) =		26556.00			
Runoff Coefficient, C (5 yrs) =		0.86			
Rain fall Data for the site from BOM					
Storm Duration T (min)	I 5 yrs (mm/hr)	V of Hydrograph (m³)	Inflow Peak discharge (L/s)		
1	185	70.42	1173.63		
2	143	108.86	907.18		
3	135	154.16	856.43		
4	129	196.41	818.37		
5	124	235.99	786.65		
10	101	384.44	640.74		
15	84.2	480.74	534.16		
20	72.1	548.88	457.40		
25	63.2	601.40	400.94		
30	56.3	642.89	357.16		
45	43	736.53	272.79		
60	35.3	806.19	223.94		
90	26.7	914.67	169.38		
120	22	1004.88	139.57		
180	17	1164.75	107.85		
Q	1173.63	L/s			
V	6	m/s (should be from 0.6 to 6 m/s)			
Q=VA					
A (Full)	0.195604611				
d Full)	499.0506247	mm	< 900 mm	Existing pipe from site to Council	
			OK		
Mannings n for RC pipe	0.013				
Existing pipe dia	900	mm			
Pipe length	11.4	m			
Upstream IL	15.36	m			
Downstream IL	14.71	m			
Hydraulic Radius, R (Full Flow)	225	mm			
Slope, S	0.057017544				
Pipe flow Velocity	6.794892325	m/s, By Manning's formula			

Note: Area drained to site storage must now be 85% not 75% as shown in this exampleⁱ

DRAINAGE DESIGN SUMMARY		SUB/DA No. _____	
Project: TRADEMASTER		Location: 46-52 Ferndell St, South Granville	
Designed by: Ertaz Chowdhury		Company: Australia Wide Consulting Phone: _____ Services P/L	
SITE AREA <u>2.657</u> ha [A]			
*See Section 4.1.5 for dual occupancy			
Basic storage volume	$470 \times [A] \text{ 2.657 }$	$= \text{ 1248.79 } \text{ m}^3$	[B]
Basic discharge	$= 0.08 \times [A] \text{ 2.657 }$	$= \text{ 0.21256 } \text{ m}^3/\text{s}$	[C]
Area of site drained to storage (Must be as much as possible and not be less than 75% of the total site without written Council approval).		$= \text{ 2.2665 } \text{ ha}$	[D]
$[D] / [A] + [\text{ 2.2665 }] / [\text{ 2.657 }] \times 100$		$= \text{ 85.3 } \%$	[E]
Storage per ha. of contributing area $= [B] / [D]$		$= \text{ 550.98 }$	[F]
Enter volume/PSD adjustment chart (Fig 5.1) using [F], and Read new PSD in litres/second/ha.		$= \text{ 66.3 } \text{ l/s/ha}$	[G]
Determine PSD $= [G] \times [D] \text{ 66.3 } \times \text{ 2.2665 }$		$= \text{ 150.27 } \text{ l/s}$	[H]
Maximum head to orifice centre		$= \text{ 1.60 } \text{ m}$	[K]
Weir flow to storage $Q^{\text{Weir}} = CL(H^{\text{Weir}})^{1.5} \therefore H^{\text{Weir}}$		$= \text{ 0.15 } \text{ m}$	[I]
Selected orifice diameter: $d = (0.464 \times Q / \sqrt{h})^{0.5} = (0.464 \times [H] / \sqrt{[K]})^{0.5}$		$= \text{ 0.24 } \text{ m}$	[J]
Maximum discharge		$= \text{ 150.27 } \text{ l/s}$	[L]
Head for high early discharge		$= \text{ 1.45 } \text{ m}$	[M]
High early discharge $\{ [L] \times \sqrt{[M] / [K]} \}$ (min 75% of [L])		$= \text{ 143.05 } \text{ l/s}$	[N]
Approximate mean discharge $= ([L] + [N]) / 2$		$= \text{ 146.66 } \text{ l/s}$	[P]
Average discharge/ha $= [P] / [D] = \text{ 146.66 } / \text{ 2.2665 }$		$= \text{ 64.71 } \text{ l/s/ha}$	[Q]
Enter volume/P.S.D. adjustment chart (Fig 5.1) using [Q] And read off final storage volume per hectare		$= \text{ 552.42 } \text{ m}^3/\text{ha}$	[R]
Determine final SSR $= [R] \times [D] = \text{ 552.42 } \times \text{ 2.2665 }$		$= \text{ 1252.06 } \text{ m}^3$	[S]
Primary storage proportion $= [S] \times \text{ _____ } \%$		$= \text{ _____ } \text{ m}^3$	[T]
Secondary storage proportion $= [S] \times \text{ _____ } \%$		$= \text{ _____ } \text{ m}^3$	[U]
Tertiary storage proportion $[S] \times \text{ _____ } \%$		$= \text{ _____ } \text{ m}^3$	[V]
Check $[T] + [U] + [V] = [S]$		$= \text{ _____ } \text{ m}^3$	
Orifice diameter 200mm used based on 112.92 l/s discharge rate from STORMFILTER			

SIGNATURE: _____ DATE: 08/05/2024

ⁱ Area draining changed in revision 2 of third edition



STORMFILTER DESIGN TABLE

- STORMFILTER TREATMENT CAPACITY VARIES BY NUMBER OF FILTER CARTRIDGES INSTALLED.
- THE STANDARD CONFIGURATION IS SHOWN. ACTUAL CONFIGURATION OF THE SPECIFIED STRUCTURE(S) PER CERTIFYING ENGINEER WILL BE SHOWN ON SUBMITTAL DRAWING(S).
- FILTER CARTRIDGES SHALL BE MEDIA-FILLED, PASSIVE, SIPHON ACTUATED, RADIAL FLOW, AND SELF-CLEANING. RADIAL MEDIA DEPTH SHALL BE 178mm.

CARTRIDGE NAME / SIPHON HEIGHT (mm)	690	460	310
CARTRIDGE PHYSICAL HEIGHT (mm)	840	600	600
TYPICAL MINIMUM WEIR HEIGHT [H] (mm)	920	690	540
CARTRIDGE FLOW RATE FOR ZPG MEDIA (L/s)	1.6	1.1	0.7
CARTRIDGE FLOW RATE FOR PSORB MEDIA (L/s)	0.9	0.46	0.39

STORMFILTER CARTRIDGE DETAIL

SITE SPECIFIC DATA REQUIREMENTS	
STRUCTURE ID	[]
NUMBER OF CARTRIDGES REQ'D	[84]
SIPHON HEIGHT (310 / 460 / 690)	[690]
MEDIA TYPE (ZPG / PSORB)	[60]
WATER QUALITY FLOW RATE (L/S)	[0.9]
DIMENSION A	[14.14m]
DIMENSION B	[7.05m]
TOTAL CARTRIDGE BAY AREA (A x B) TO MATCH AREA REQUIRED BY MUSIC MODELLING OR COUNCIL SPECIFIC REQUIREMENTS	

GENERAL NOTES

- INLET AND OUTLET PIPES TO BE IN ACCORDANCE WITH APPROVED PLANS.
- A HIGH FLOW BYPASS ARRANGEMENT OR DISSIPATION STRUCTURE MAY BE REQUIRED TO MINIMISE RE-SUSPENSION OF SOLIDS OR ANY SIGNIFICANT INERTIAL FORCES ON THE CARTRIDGES.
- ALL WATER QUALITY TREATMENT DEVICES REQUIRE PERIODIC MAINTENANCE. REFER TO OPERATION AND MAINTENANCE MANUAL FOR GUIDELINES AND ACCESS REQUIREMENTS.
- SITE SPECIFIC PRODUCTION DRAWING WILL BE PROVIDED ON PLACEMENT OF ORDER.
- THE INVERT LEVEL OF THE INLET PIPE MUST BE GREATER THAN THE RL OF THE FALSE FLOOR WITHIN THE CARTRIDGE CHAMBER.
- CONCRETE STRUCTURE AND ACCESS COVERS DESIGNED AND PROVIDED BY OTHERS. ACCESS COVERS TO BE A MINIMUM 900 X 900 ABOVE CARTRIDGES. OH&S REGARDING ACCESS COVERS AND TANK ACCESS TO BE ASSESSED BY OTHERS ON SITE.
- THE STRUCTURE THICKNESSES SHOWN ARE FOR REPRESENTATIONAL PURPOSES.
- DRAWINGS NOT TO SCALE.

INSTALLATION NOTES

- UNDERDRAIN AND FALSE FLOOR INSTALLED BY OCEAN PROTECT.

PHONE: 1300 354 722 www.oceanprotect.com.au

OCEAN PROTECT

SINGLE ORIFICE WITH A HED WEIR

STORMFILTER DETENTION TANK

CUMBERLAND COUNCIL SPECIFICATION DRAWING

StormFilter Flow Calculator - Psorb Media

The equation below defines the relationship between the diameter of the orifice plate, the flow rate through the cartridge and head.

$$Q = (0.111d^2 \cdot 2.06h^{0.5}) / 60$$

where d = Restrictor Disc Diameter
Δh = head

Cartridge Name

690

Cartridge Quantity

84

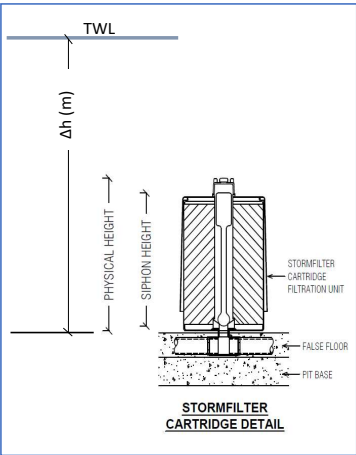
Δh (m)

1.71

Total Q at head

112.92





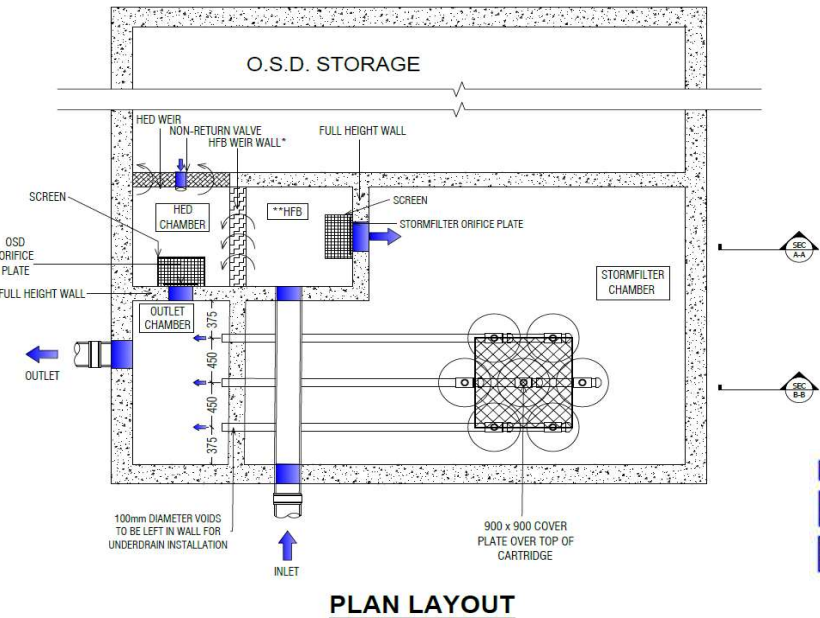
Design Process (Parramatta Council)

- Step 1: Read through the design process

Step 2: Configure your tank to suit

Step 3: Complete the calculation steps

Step 4: Produce the data table and copy the table above
- Incoming flow connects to a HFB (High Flow Bypass) pit or sometimes called an inlet chamber.
 - The HFB is ≤ 1m2.
 - An orifice diverts up to the 3mth flow to the SF (StormFilter) chamber based on the available head in the HFB i.e. OSD TWL to SF orifice centreline.
 - The floor of the HFB (equivalent to the 3mth orifice centreline) is to be approx. 150mm above the false floor in the SF chamber.
 - The HFB overflows to the HED.
 - The HFB overflow weir (which is essentially the SF weir) is to be equal or greater than the HED weir. If the HED weir is lower than the minimum SF driving head, the SF weir needs to be adopted.
 - The SF chamber should hold the 3mth flow volume (based on 43.4m/hr intensity). The chamber dimensions are therefore to be based on the available water depth.
 - All walls around the SF to be at the 100yr TWL.
 - The StormFilter underdrains are free draining and connect to the Overflow pit downstream of the HED chamber. A calculation needs to be performed to determine the max flow through the cartridges during the 100yr storm.
 - The HED orifice has been adjusted to account for the flow through the StormFilters at the 100yr event.



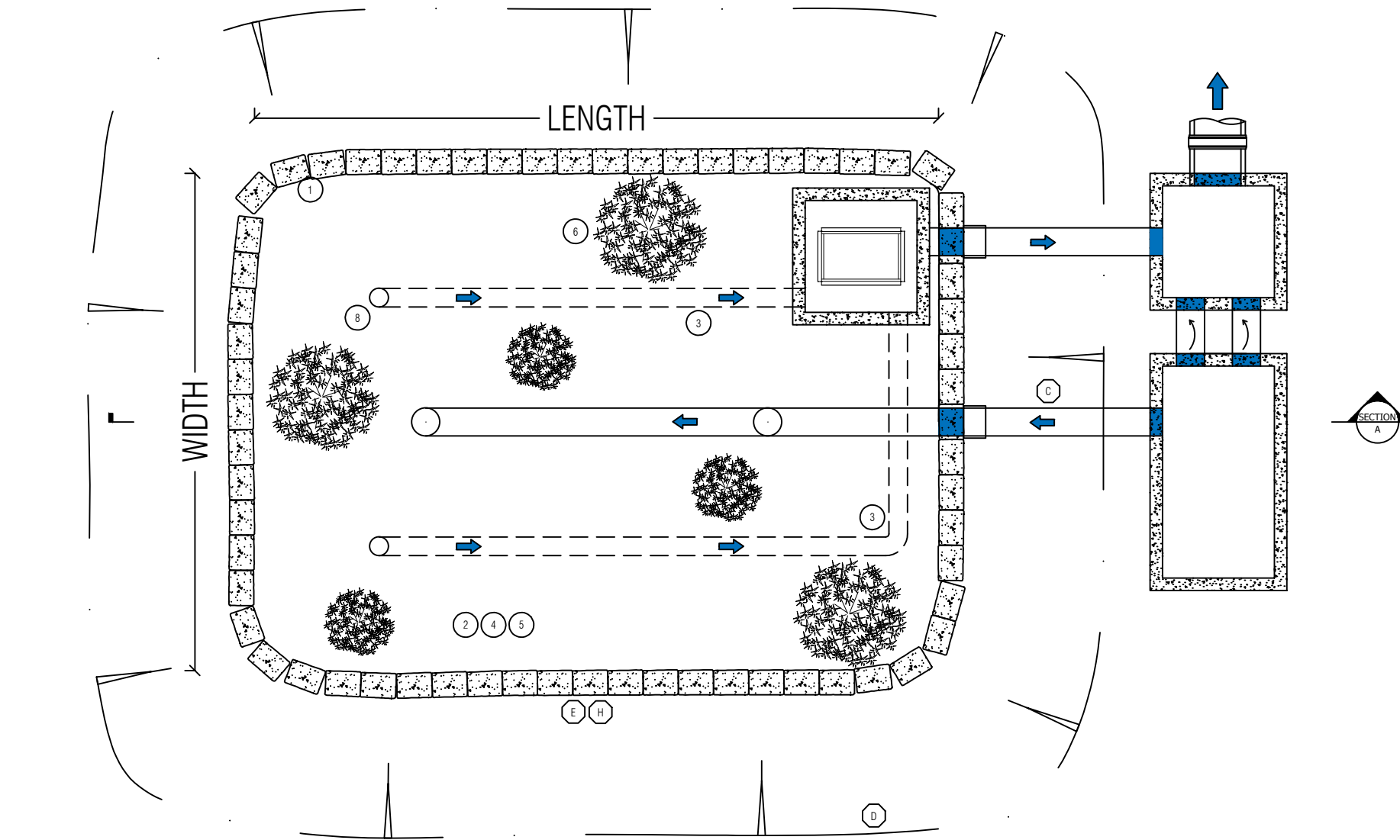
PLAN LAYOUT

- MUSIC High Flow Bypass
- Peak Flow through Cartr. use spreadsheet
- Simple Rational Calc 3mth Intensity is 43.4mm/hr

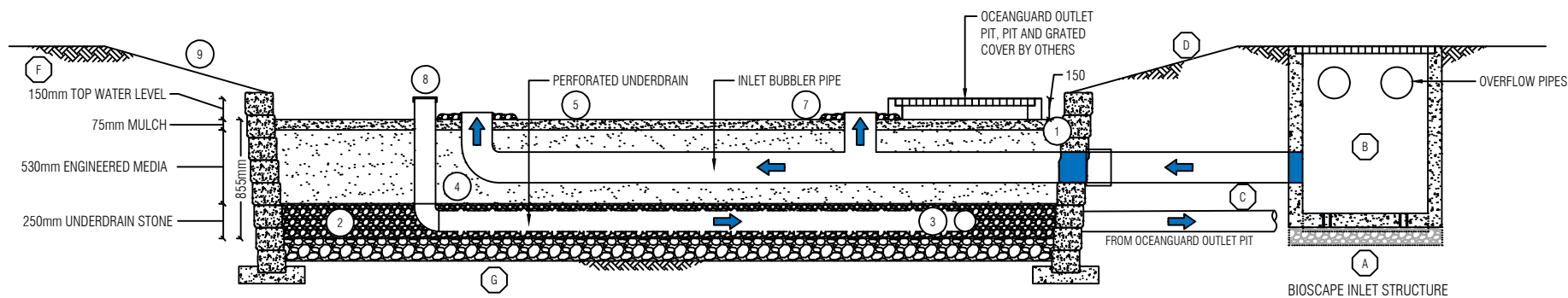
SITE SPECIFIC DATA REQUIREMENTS	
STRUCTURE ID	100yr TWL
WATER QUALITY FLOW RATE (L/S)	100%
PEAK FLOW RATE (L/S)	11.15L/s
RETURN PERIOD OF PEAK FLOW (yrs)	100yr (100/100)
# OF CARTRIDGES REQUIRED	0
CARTRIDGE HEIGHT (310, 460 or 690mm)	690
MEDIA TYPE (PERLITE, PERLITE/ZEOLITE OR ZPG)	PERLITE

* HFB WEIR WALL HEIGHT GREATER THAN OR EQUAL TO THE HED WEIR HEIGHT EXCEPT WHEN THE HED WALL HEIGHT IS LOWER THAN IS REQUIRED FOR PROPER FUNCTION OF STORMFILTERS

**HFB - HIGH FLOW BYPASS CHAMBER
NO GREATER THAN 1M X 1M



PLAN



SECTION A-A

(PLANTING NOT SHOWN FOR CLARITY)

SITE SPECIFIC REQUIREMENTS		
COUNT	DESCRIPTION	INSTALLED BY
5m ² +9m ²	FILTERRA SURFACE AREA (m ²)	OCEAN PROTECT
	MULCH VOLUME (m ³)	OCEAN PROTECT
	FILTERRA MEDIA DEPTH (mm)	OCEAN PROTECT
	VOLUME OF UNDERDRAIN STONE (m ³)	OCEAN PROTECT
	FILTERRA LINER (m)	OCEAN PROTECT

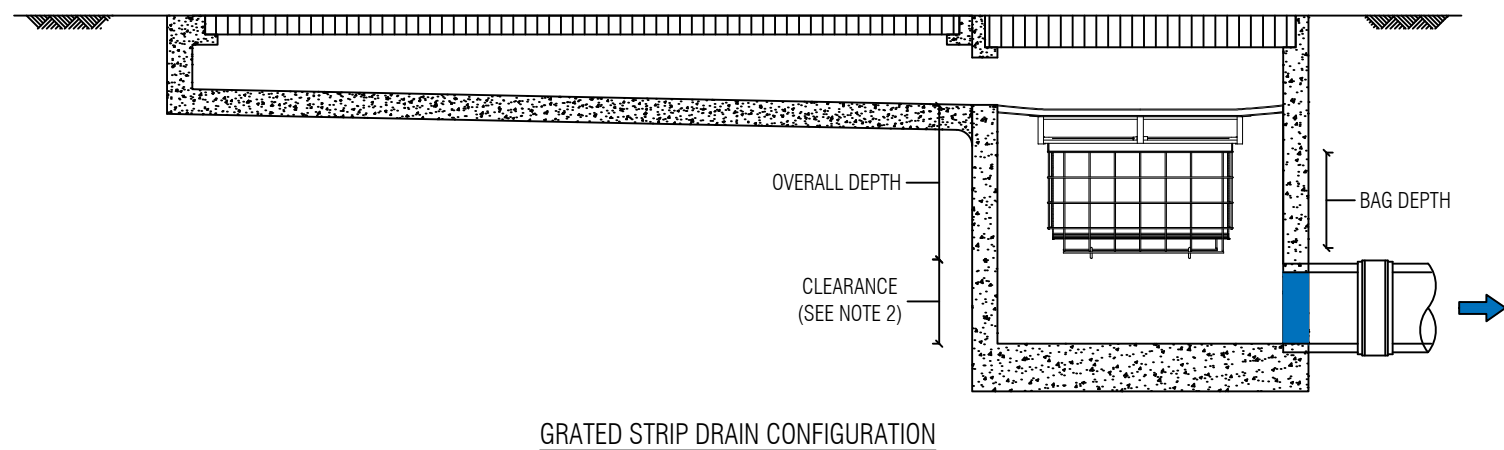
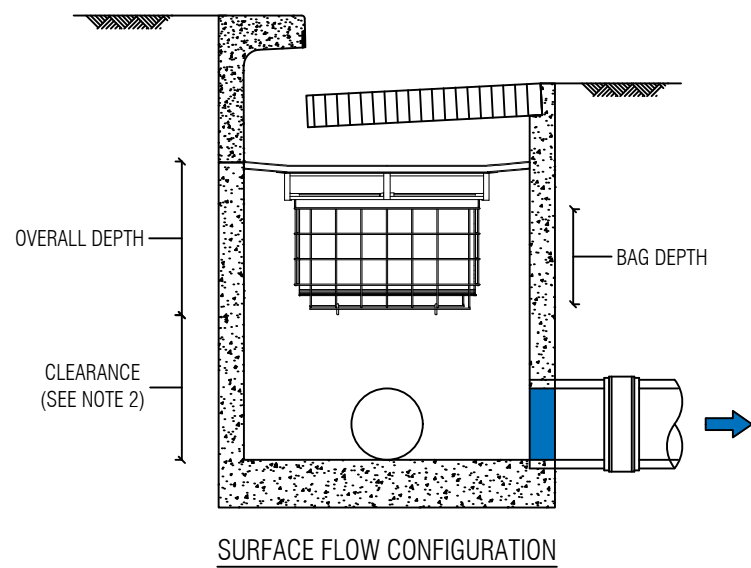
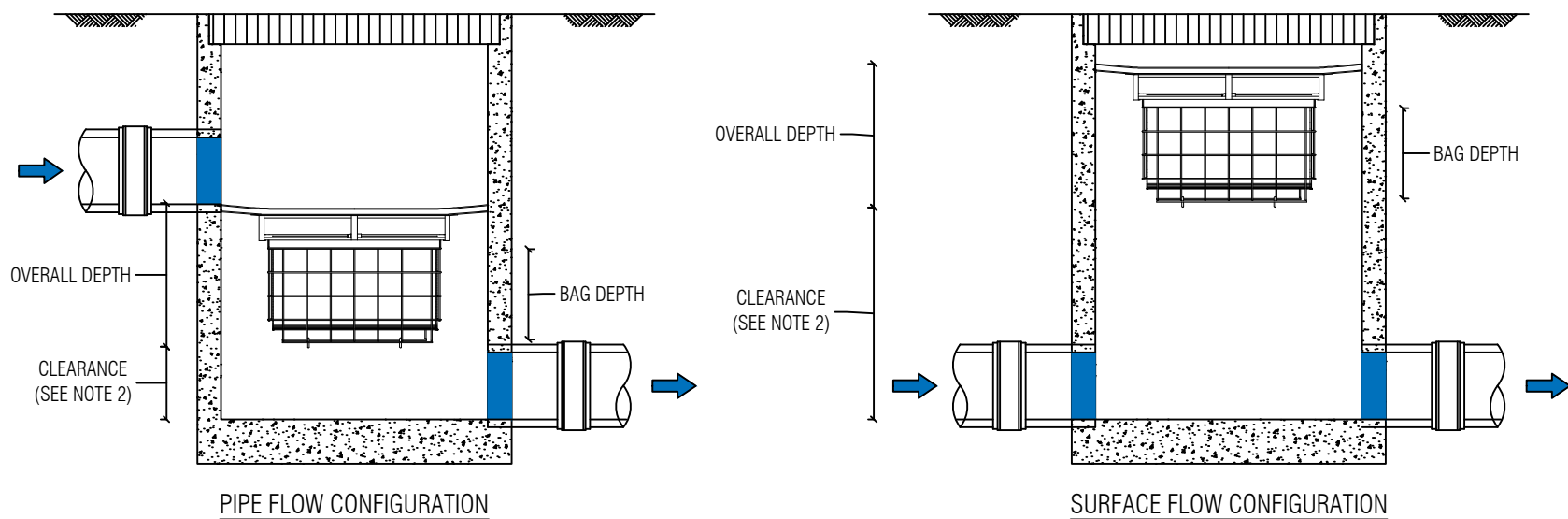
PLANTING SCHEDULE	
COUNT	FILTERRA BIOSCAPE SYSTEM PLANT PALETTE

- GENERAL NOTES**
- CONTRACTOR SHALL CONTACT OCEAN PROTECT TO COORDINATE DELIVERY AND INSTALLATION OF FILTERRA BIOSCAPE SYSTEM. OCEAN PROTECT ACTIVATION CAN ONLY OCCUR ONCE CONTRACTOR RESPONSIBILITIES ARE COMPLETE.
 - PERFORM FILTERRA BIOSCAPE SYSTEM EXCAVATION ONLY AFTER ALL THE CONTRIBUTING DRAINAGE AREAS ARE PERMANENTLY STABILISED. IF FILTERRA BIOSCAPE SYSTEM IS IN AN AREA PREVIOUSLY USED AS EROSION AND SEDIMENT CONTROL FACILITIES PLEASE CONTACT OCEAN PROTECT. DO NOT STOCKPILE MATERIALS OR STORE EQUIPMENT IN THIS AREA. CONTRACTOR SHALL TAKE APPROPRIATE MEASURES TO PREVENT CONSTRUCTION-RELATED EROSION RUNOFF FROM ENTERING THE FILTERRA MEDIA BAY.
 - FILTERRA SHALL BE INSTALLED OFFLINE AS EARLY AS POSSIBLE AFTER SITE STABILISATION TO ALLOW FOR SOIL MATURITY AND SYSTEM ESTABLISHMENT.
 - CONTRACTOR SHALL COORDINATE WITH OCEAN PROTECT BEFORE THE FILTERRA BIOSCAPE SYSTEM IS EXCAVATED TO MINIMISE THE TIME BETWEEN EXCAVATION AND COMPLETION OF THE FILTERRA BIOSCAPE SYSTEM. ONCE EXCAVATED, ANY STANDING WATER THAT ACCUMULATES IN THE EXCAVATED AREA MUST BE REMOVED BY THE CONTRACTOR BEFORE OCEAN PROTECT CAN COMMENCE THE FILTERRA BIOSCAPE SYSTEM. ANY ADDITIONAL EXCAVATION WILL BE THE RESPONSIBILITY OF THE CONTRACTOR.
 - CONTRACTOR SHALL PROVIDE ACCESS TO THE EXCAVATED AREA(S) FOR OCEAN PROTECT TO USE DURING THE CONSTRUCTION OF THE FILTERRA BIOSCAPE SYSTEM(S). ACCESS SHALL NOT PROHIBIT LIGHT DUTY EQUIPMENT THAT MAY BE USED TO INSTALL THE COMPONENTS (STONE, MEDIA, ETC). THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY RE-STABILIZATION THAT MAY BE REQUIRED AFTER THE FILTERRA BIOSCAPE SYSTEM INSTALLATION/ACTIVATION.
 - OCEAN PROTECT AND/OR ITS REPRESENTATIVES SHALL BE RESPONSIBLE FOR THE LIST ENTITLED 'OCEAN PROTECT INSTALLATION RESPONSIBILITIES'.
 - NO FLOW SHALL ENTER THE FILTERRA SYSTEM UNLESS OCEAN PROTECT HAS ACTIVATED THE SYSTEM AND CONFIRMED ESTABLISHMENT
 - IF FILTERRA IS WITHIN AN OVERLAND FLOW PATH, PLEASE CONTACT OCEAN PROTECT

- CONTRACTOR WORKS AND INSTALLATION RESPONSIBILITIES**
- A. CONTRACTOR TO PLACE FILTERRA INLET STRUCTURE ON BEDDING AS SPECIFIED BY THE ENGINEER. OCEAN PROTECT SUGGESTS AS A MINIMUM TO USE 150MM BEDDING STONE ON COMPACTED SUB-GRADE TO 90% DENSITY. UNSUITABLE MATERIAL SHALL BE REPLACED AS ADVISED BY THE ENGINEER.
- B. CONTRACTOR SHALL PROVIDE AND INSTALL DRAINAGE ITEMS TO, FROM AND INCLUDING THE INLET AND OUTLET STRUCTURES AS PER THE APPROVED SITE PLANS.
- C. OCEAN PROTECT CAN PROVIDE COUPLERS AT THE FILTERRA INTERFACE FOR CONNECTION TO THE INLET DIVERSION PIPES. ALL DRAINAGE TO AND FROM THE FILTERRA MUST ALLOW FOR POSITIVE FLOW.
- D. CONTRACTOR TO PROVIDE BATTER ACCORDING TO DIMENSION AND SLOPE SHOWN ON PLANS. SLOPE FROM SHOULDER TO FILTERRA BIOSCAPE SYSTEM SURFACE PERIMETER SHALL NOT EXCEED 3:1. TURF IS REQUIRED TO STABILISE SIDE SLOPES SHOWN ON DETAIL AND ON PLAN SHEETS.
- E. CONTRACTOR TO EXCAVATE MEDIA AREA CORRESPONDING TO THE SIZE OF THE FILTERRA BIOSCAPE SYSTEM SURFACE AREA AS SHOWN ON DETAIL AND ON PLAN SHEETS.
- F. CONTRACTOR SHALL EXCAVATE VERTICALLY FROM BOTTOM OF UNDERDRAIN STONE OR DRAINAGE STONE IF REQUIRED, TO ELEVATION OF MULCH AS SHOWN ON THIS DETAIL.
- G. CONTRACTOR TO CONFIRM DEPTH OF EXCAVATION. IF THE EXCAVATION HAS BEEN MADE TOO DEEP AND ADDITIONAL UNDERDRAIN STONE NEEDS TO BE USED TO RAISE THE BASE OF THE FILTERRA, THIS COST SHALL BE TAKEN ON BY THE CONTRACTOR.
- H. RETAINING WALLS AND ADDITIONAL EROSION CONTROL AROUND THE FILTERRA BIOSCAPE SYSTEM. RETAINED OFFLINE FROM FILTERRA OCEAN PROTECT SUPPLY AND INSTALLATION RESPONSIBILITIES

- GEOTEXTILE FABRIC ALONG THE PERIMETER OF THE FILTERRA BIOSCAPE SYSTEM EXCAVATION.
- UNDERDRAIN STONE. TYPICALLY 250mm THICK (50mm UNDER THE PIPING 150mm AROUND THE PIPING AND 50mm ABOVE THE PIPING)
- 150mm UNDERDRAIN PIPING UNLESS OTHERWISE APPROVED BY OCEAN PROTECT, ASSOCIATED PIPING AND FITTINGS/ELBOWS TO CONNECT TO THE PIPING/FITTING(S) THAT IS PROVIDED BY CONTRACTOR (SEE CONTRACTOR INSTALLATION RESPONSIBILITIES THIS DETAIL).
- 530mm FILTERRA MEDIA.
- 75mm DOUBLE SHREDDED HARDWOOD MULCH OVER ENTIRE FILTERRA BIOSCAPE SYSTEM SURFACE AREA.
- PLANTINGS OF PLANTS SELECTED BY ENGINEER THAT MATCH OCEAN PROTECTS APPROVED PLANTING LIST.
- FLOW DISSIPATION APRON AROUND ALL FLOW ENTRY POINTS AS DESIGNED AND INDICATED ON THIS DETAIL
- CLEAN-OUT ADAPTER, PLUG AND PIPING.
- ACTIVATING THE SYSTEM





PLAN ID	MAXIMUM PIT PLAN DIMENSIONS
S	450mm x 450mm
M	600mm x 600mm
L	900mm x 900mm
XL	1200mm x 1200mm

DEPTH ID	BAG DEPTH	OVERALL DEPTH
1	170	270
2	300	450
3	600	700

PLAN ID	DEPTH ID			
	S	1	2	3
	M	■	■	■
	L	■	■	■
	XL	■	■	■



GENERAL NOTES

1. THE MINIMUM CLEARANCE DEPENDS ON THE CONFIGURATION (SEE NOTE 2) AND THE LOCAL COUNCIL REQUIREMENTS.
2. CLEARANCE FOR ANY PIT WITHOUT AN INLET PIPE (ONLY USED FOR SURFACE FLOW) CAN BE AS LOW AS 50mm. FOR OTHER PITS, THE RECOMMENDED CLEARANCE SHOULD BE GREATER OR EQUAL TO THE PIPE OBVERT SO AS NOT TO INHIBIT HYDRAULIC CAPACITY.
3. OCEAN PROTECT PROVIDES TWO FILTRATION BAG TYPES:- 200 MICRON BAGS FOR HIGHER WATER QUALITY FILTERING AND A COARSE BAG FOR TARGETING GROSS POLLUTANTS.
4. DRAWINGS NOT TO SCALE.



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TYPICAL ARRANGEMENTS
SPECIFICATION DRAWING